OFFK	CE OF THE STATE ENGINEER - DIVISION OF WATER RES JES - DAM SAFETY BRANCH 1313 SHERMAN ST. , ROOM B18, DENVER, CO 80203.		6-35	DG 31	M
DAM CLAS DIV: EPP: <u>CUI</u>	SS: 3 DAM LENGTH(FT): 675.0 SPILLWAY CAPACITY(CFS): 5940.0 CAPACITY(AF): 3 WD: 21 CRESTWIDTH(FT): 20.0 FREEBOARD (FT): 9.0 SURFACE AREA(AC) : Not Required CRESTELEV(FT): 11230.0 DRAINAGE AREA (AC.): 2560.0 OUTLET INSPECTED RRENT RESTRICTION NONE	DN:	10 27	1997	2005
ADD	NER: COLORADO DEPT OF PUBLIC HEALTH & ENVIRONM CONTACT NAME: AUSTIN BUCKINGHAM DRESS: MC: HMD-RP-B2 CONTACT PHONE: (303) 692-3435 DENVER CO 80246 PRESENTING : CDPHE Dennis Witte RTG/Golder Assoc. RTG/Golder Assoc. RTG/Golder Assoc.				
1000	NDITIONS WATER LEVEL: BELOW DAM CREST 10.75 FT. Below Spillway 1.75 FT. GAGE ROD READING	1^	1219	.75	
OBS	SERVED GROUND MOISTURE CONDITION: DRY WET SNOWCOVER OTHER SUNNY, COOL, breed	zy, dr	-	nditic	ons
UPSTREAM SLOPE	PROBLEMS NOTED (0)NONE (1)RIPRAP - MISSING, SPARSE, DISPLACED, WEATHERED (2) WAVE EROSION - WITH SCARPS (3) CRACKS WITH DISPLACEMENT (4) SINKHOLE (5) APPEARS TOO STEEP (6) DEPRESSIONS OR BULGES (7) SLIDES (8) CONCRETE FACING - HOLES, CRACKS, DISPLACED, UNDERMINED (9) OTHER Generally good coverage of angular hard rock riprap on exposed portion of slope; some areas of soil mixed i with riprap. Fairly uniform slope overall. (1) Some weathering of scattered softer rock pieces along slope, due		-	A C C E P T A B L	
CREST	to severe (cont. pg 3) PROBLEMS NOTED (10) NONE (11) RUT OR PUDDLES (12) EROSION (13) CRACKS - WITH DISPLACEMENT (14) SINKHOLES (15) NOT WIDE ENOUGH (16) LOW AREA (17) MISALIGNMENT (18) IMPROPER SURFACE DRAINAGE (19) OTHER Good width, which varies from 20 feet nominal to about 40 feet on the right end turnaround area. Good granular surfacing. Elevation is fairly uniform, except for built up area around generator unit, left of outlet section, which is about (cont. pg 3)		GOOD	E X A C C E P T A B L E	a o o d CREST
DOWNSTREAM SLOPE	PROBLEMS NOTED (20) NONE (21) LIVESTOCK DAMAGE (22) EROSION OR GULLIES (23) CRACKS - WITH DISPLACEMENT (24) SINKHO ✓ (25) APPEARS TOO STEEP (26) DEPRESSIONS OR BULGES (27) SLIDE (28) SOFT AREAS (29) OTHER Very hard, well-compacted rocky soil surface, with no vegetative cover, but no evident erosion problems. Fairly uniform appearance. Access road from left end of crest angles downward across slope to right abutment spillway crossing. (cont. pg 3)	this Sheet	G O O D	X ACCEPTABLE	DOWNSTREAM SLOPE
SEEPAGE	PROBLEMS NOTED (30) NONE ✓ (31) SATURATED EMBANKMENT AREA ✓ (32) SEEPAGE EXITS ON EMBANKMENT ✓ (33) SEEPAGE EXITS AT POINT SOURCE ✓ (34) SEEPAGE AREA AT TOE ✓ (35) FLOW ADJACENT TO OUTLET (36) SEEPAGE INCREASED / MUDIP DRAIN OUTFALLS SEEN ✓ No Yes Show location of drains on sketch and indicate (37) FLOW INCREASED / MUDIP (38) DRAIN DRY / OBSTRUCTE ✓ (39) OTHER Abutment seepage; spillway seepage Dam has no known drain outfall pipes. Reservoir filled from 6' above empty this spring, peaking at G.H. 11220.0 six days prior to this inspection, & has been drawndown only 3'' since. (31)(32) A previously unnoted area of wetness and minor (cont. pg 3)	Guidelines on Back of	GOOD	A C C E P T A B L E	2004 SEEPAGE
	PROBLEMS NOTED (40) NONE (41) NO OUTLET FOUND (42) POOR OPERATING ACCESS (43) INOPERABLE	See	F	x	
OUTLET	 (44) UPSTREAM OR DOWNSTREAM STRUCTURE DETERIORATED (45) OUTLET OPERATED DURING INSPECTION YES NO INTERIOR INSPECTED (120) NO (121)YES (46) CONDUIT DETERIORATED OR COLLAPSED (47) JOINTS DISPLACED (48) VALVE LEAKAN (49) OTHER Gate corrosion Dam has a pair of upper-level gravity outlet conduits located near the right end of the dam and positioned with their inverts about 13 feet below the spillway crest elevation. Both outlets are controlled by upstream slide gates activated by (cont. pg 3) 	Num-	GOOD	A C C E P T A B L E	BOOR .
SPILLWAY	PROBLEMS NOTED (50) NONE (51) NO EMERGENCY SPILLWAY FOUND (52) EROSION WITH BACKCUTTING (53) CRACK - WITH DISPLACEMENT (54) APPEARS TO BE STRUCTURALLY INADEQUATE (55) APPEARS TOO SMALL (56) INADEQUATE FREEBOARD (57) FLOW OBSTRUCT (58) CONCRETE DETERIORATED / UNDERMINED (59) OTHER Spillway consists of a trapezoidal open channel cut through rock on the right abutment, with a concrete cress section having a length of 61 feet. This spillway was reportedly designed to pass the 50% PMP (cont. pg 4)	D	G O O D	X ACCEPTABLE	SPILLWAY

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DAM NAME	SUMMITVILLE TAILINGS	Page 2	DAM I.D.: 1 03	DATE.	7/14/2005		
U EXISTI		(111) GAGE ROD (112) PIEZOMETE	RS (113) SEEPAGE WEIRS	/ FLUMES	X X 9		
a (114	4) SURVEY MONUMENTS (115) OTHER				G C P C		
MONIT	ORING OF INSTRUMENTATION (116) NO	(117) YES PERIODIC INSPECTIONS BY:	(118) OWNER (119) ENGINE	ER	T C C C C C C C C C C C C C C C C C C C		
) Pressure transducer in reservoir		3) At toe pond at maxim	um	D TABLE		
E sect	ion. (117)(118) Site operator perso	onnel on site on a regular basis.		一部一部的影	i z		
PROBL	LEMS NOTED: 660 NONE 661) ACCESS F	ROAD NEEDS MAINTENANCE (62) CATT	LE DAMAGE		x		
	BRUSH ON UPSTREAM SLOPE, CREST. DOWN	STREAM SLOPE, TOE (64) TREES ON UPS	TREAM SLOPE, CREST, DOWNSTR	EAM SLOPE, TOE	G A P ON		
	RODENT ACTIVITY ON UPSTREAM SLOPE, CRE	ST. DOWNSTREAM SLOPE, TOE 🗌 (66) DETE	RIORATED CONCRETE - FACING, C	UTLET SPILLWAY			
H C (67)) GATE AND OPERATING MECHANISM NEED MAIN	TENANCE (68) OTHER					
<u>(67)</u>	As described above.						
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	<u>i impounds reservoir of site runof</u> ot a statutorily exempt structure. I			ew indicates that	<u>it dam</u> ្រុប		
AS		Same an generally good contaits			<u>i</u> ğr		
E D					DIT		
Based	on this Safety Inspection and recent file review	, the overall condition is determined to be:			60		
✓ (71)) SATISFACTORY	2) CONDITIONALLY SATISFACTORY	(73) UNSATISF	ACTORY	Ū		
	r	TEMS REQUIRING ACTION BY OWN	ER	the second s			
e de		O IMPROVE THE SAFETY OF THE D	AM				
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inspection report, does no f the subject dam. The sol the reservoir owner or op t damages caused by leaka esulting from a failure of th		AND PROPERLY BACKFILL EXISTING HOLES:					
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specti the sul the res damag sulting	(85) PROVIDE SURFACE DRAINAGE FOR:						
コロシビー	(87) DEVELOP AND SUBMIT AN EMERGENC)	, observe wet area above outlets for PREPAREDNESS PLAN:	changes in size & quantity	& quality			
dam safety i condition o m rests with y to prevent r or floods r	(88) OTHER Remove pipeline and	support piers from spillway approa	ch channel: remove log del	oris			
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0 2 2 4 5		ATION TO EVALUATE THE STABILITY OF THE DA	.M:				
r, by bility f he sa very s fror	(93) PERFORM A HYDROLOGIC STUDY TO D	ETERMINE REQUIRED SPILLWAY SIZE:					
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assu assu resp who over	(98) OTHER:	additional erosion protection in low	er spillway chainlei				
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	SAFE STORAGE	LEVEL RECOMMENDED AS A RESULT O					
	(102) CONDITIONAL FULL STORAGE	ſ	FT. BELOW DAM CREST FT. BELOW SPILLWAY CRES	т			
	(103) RECOMMENDED RESTRICTION		FT. GAGE HEIGHT	I ST SULLY ODEN			
DEMONIE	(104) CONTINUE EXISTING RESTRICTION		NO STORAGE-MAINTAIN OUT	LET FULLY OPEN			
REASON FO	JR RESTRICTION	External contract of the					
ACTIONS DO							
ACTIONS RE	EQUIRED FOR CONDITIONAL FULL STORAGE OR CO	ALINGED STORAGE AT THE RESTRICTED LEVEL:					
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Engineer's Signature	Ut Commen	Owner's Signature		DATE:	11		
	INSPECTED BY		OWNER/OWNER'S REPRESEN	TATIVE	pp2 of 4		

M NAME: SUMMITVILLE TAILINGS Page 2 DA	AMI.D.: 2 33 DATE. 7/14/2005						
	(113) SEErc WEIRS / FLUMES						
(114) SURVEY MONUMENTS (115) OTHER							
MONITORING OF INSTRUMENTATION (116) NO (117) YES PERIODIC INSPECTIONS BY: (118)							
(111) Pressure transducer in reservoir bottom. (112) On D/S slope. (113) At 1	toe pond at maximum						
section. (117)(118) Site operator personnel on site on a regular basis.							
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(67) GATE AND OPERATING MECHANISM NEED MAINTENANCE (68) OTHER							
(67) As described above.	Le Z						
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is not a statutorily exempt structure. Dam is in generally good condition.	Lainings uant. Review indicates that uant						
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Based on this Safety Inspection and recent file review, the overall condition is determined to be:							
	(73) UNSATISFACTORY						
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69 0	cifications must be approved by State Engineer prior to construction.						
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Upstream Slope (cont.)

(1)(cont.) climate conditions at high altitude site.

(5) The exposed upper portion of the slope varies somewhat in inclination, from about 1.5:1 along the maximum section, to about 2:1 at the outlet section. No signs of instability were noted.

Crest (cont.)

1.5 feet above the surrounding crest elevation.

(11) Minor.

(18) Both the left end of the crest and the turnaround area at the right end appear to slope to the downstream side, which is not desirable for erosion control purposes.

Downstream Slope (cont.)

(25) The upper part of the slope is oversteepened, to about 1.5:1, due to the presence of the access road fill across it. The lower slope inclination is more reasonable, and probably more representative of the actual dam slope, at about 2.5:1 to 3:1. Overall slope at the maximum section, measured from the downstream edge of the road fill to the toe, is 2:1.

Seepage (cont.)

- (31)(32)(cont.) seepage was observed on the downstream slope at the outlet section, just above the access roadway which angles across the slope there. The wet area was about 50 feet in length, parallel to the dam axis, and about 10 feet in width, and was producing a trickle of clear flow along the roadway ditch, flowing toward the spillway channel. The area had a mossy appearance, with some algae, but was not overly soft, being comprised of the hard rocky soil which makes up the downstream slope. This area should be watched closely for changes in size and quantity and clarity of seepage.
- (34) As noted in the past, clear seepage emerges from the rockfill toe of the dam at the maximum section, in the form of several small streams of several gpm each, discharging into a pond along the valley floor beyond the toe of the dam. This pond is about 20 feet wide by about 60 feet in length, and retains water to a depth of up to 2 feet. At the lower end of the pond, most of the discharge is routed through a 4" flume for measurement; head on the flume was measured at 0.12', or about 22 gpm. Another 5 10 gpm escapes from the pond to the left of the flume without being measured. The total amount of the flow from the pond was consistent with last year's observation with the reservoir at a similar level. Flow from the pond includes seepage from the lower right abutment, as described below.
- (33)(39)(a) As noted in the past, a point source of seepage emerges from the lower right abutment near the maximum section, about 6 feet to the right of the groin and about 20 vertical feet above the toe of the maximum section. This source of seepage was producing a clear flow of 2 3 gpm, which is consistent with past observations, with soft, wet ground conditions surrounding the seep area and extending downstream along the lower abutment for 20 30 feet. Flow from the seep area discharges across a bench fill on the lower right abutment, wetting it, before discharging down the slope of the bench fill to the seep pond described above.
- (35) As previously observed, minor seepage emerges from beneath both outlet conduits at their downstream ends; this amounted to 1 -2 gpm from under the right conduit and about ¼ gpm from under the left. At the point where the outlet channel meets the spillway channel downstream of the dam, total seepage in the bottom of the outlet channel was estimated at about 5 gpm.
- (39)(b) Minor seepage emerges along the rock cut of the spillway channel, in the upper right abutment. The flow originates as trickles of seepage from the rock the right side of the spillway crest structure and at the downstream end of the left spillway wing wall. By the point where the spillway channel crosses the access roadway downstream, seepage along the spillway floor amounts to less than 5 gpm, due to small contributions of flow from the rock floor and right cut slope. By the lower end of the spillway channel, where it drops into the outlet channel, seepage flow has increased to about 10 gpm.

Outlet (cont.)

handwheels at the top of the upstream slope and inclined stainless steel stems mounted to a concrete support structure on the upstream slope. This outlet system is seldom used. The main, and preferred, method of withdrawing water from the reservoir is via a pumped outlet system, now powered by an REA electrical line from Del Norte. The intake is located on a floating barge in the reservoir, and water is conveyed to an on-site treatment plant, where it is treated to meet water quality standards before being released downstream via the Wightman Fork diversion channel, which passes around the north side of the reservoir.

Outlet (cont.)

- (45) The gravity outlet gates were closed at the time of this inspection, and were not operated. Joe Fox indicated that the gates were exercised full range and lubricated last summer when the reservoir was drawndown below the gates, and that both worked acceptably. The pumped outlet was in operation during this inspection, as usual, drawing 1010 gpm to the treatment plant, slightly more than the 900 gpm entering the reservoir from site runoff.
- (49) Corrosion of the cast iron gates and steel gate frames of the gravity outlet system has been a significant problem, due to the very low pH water retained in the reservoir. The gates and frames were coated with an epoxy paint several years ago to help retard the corrosion. Joe indicated that the gates were examined during last year's drawdown of the reservoir, and found to be in no worse condition than when the epoxy coating was applied.

Spillway (cont.)

- flood, and was constructed in 1993-94. The rock channel turns hard to the left at its lower end, and joins the outlet channel at a location about 200 feet downstream of the end of the outlet conduits. Large riprap is used to armor the outside of the curve to the outlet channel, and the area of the outlet channel where the spillway flow would enter it, but the outlet channel beyond that point is a steeply-sloping trapezoidal cut about 15 feet deep through soil to the natural stream channel of the Wightman Fork below the toc of the dam. This lower end of the spillway channel would likely suffer significant erosion during a large discharge event. However, operation of the spillway is avoided under normal operating conditions, to avoid releasing untreated water from the reservoir downstream. Recent construction of diversion channels and turnouts from the natural channels upstream of the reservoir now allow flows to be selectively diverted around the reservoir to avoid overfilling it.
- (52) Some erosion has occurred within the lower end of the spillway channel. CDPHE is considering placing additional erosion protection within the lower end of the channel to prevent further damage in the event of a large discharge event. Austin Buckingham indicated that any such work will be submitted to our office for review and approval prior to construction.
- (57) Three concrete support piers, used to support a pipeline which runs across the spillway channel, are present along the channel floor just upstream of the crest structure. These, and the support legs to the pipeline, would create some obstruction to flow. Also, a minor amount of driftwood was present around the crest structure.